

Mata Kuliah : Dinamika Struktur & Pengantar Rekayasa Kegempaan
Kode : TSP - 302
SKS : 3 SKS

Single Degree of Freedom System

Free Vibration

Pertemuan - 2

- **TIU :**

- Mahasiswa dapat menjelaskan tentang teori dinamika struktur.
- Mahasiswa dapat membuat model matematik dari masalah teknis yang ada serta mencari solusinya.

- **TIK :**

- Mahasiswa dapat memformulasikan persamaan gerak sistem struktur berderajat kebebasan tunggal yang bergetar bebas

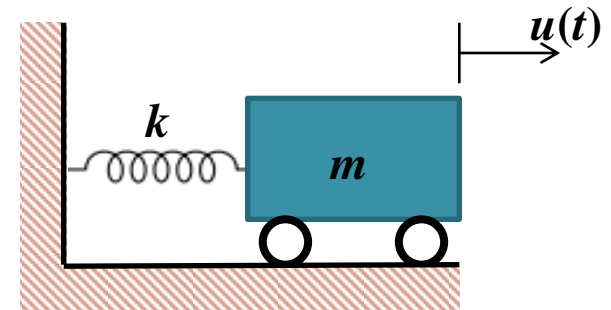
- Sub Pokok Bahasan :
 - Getaran Bebas SDoF Tak Tereadam
 - Frekuensi dan Periode
 - Getaran Bebas SDoF Dengan Redaman

Undamped SDoF Free-Vibration

- A structure is said to be undergoing free vibration when it is disturbed from its static equilibrium position and then allowed to vibrate **without** any external dynamic excitation.
- The differential equation governing free vibration of the systems without damping ($c = 0$ & $p(t) = 0$) is :

$$m\ddot{u}(t) + ku(t) = 0 \quad (1)$$

- This equation called *second order homogeneous differential equation*



- To find the solution of Eq. (1), assuming a trial solution given by :

$$u = A \cos \omega_n t \quad (2.a) \qquad u = B \sin \omega_n t \quad (2.b)$$

- Where A and B are constants depending on the initiation of the motion while ω is a quantity denoting a physical characteristic of the system.

- The substitution eq.(2.a&b) into eq. (1) gives :

$$\left(-m\omega_n^2 + k\right)A \cos \omega_n t = 0 \quad (3)$$

- If eq.(3) is to be satisfied at any time, the factor in parentheses must be equal to zero, or :

$$\omega_n^2 = \frac{k}{m} \quad \Rightarrow \quad \omega_n = \sqrt{\frac{k}{m}} \quad (4)$$

- ω_n is known as the natural circular frequency of the system (rad/s)

- Since either eq.(2.a) or (2.b) is a solution of eq.(1), and since the differential equation is linear, the superposition of these two solution is also solution :

$$u = A \cos \omega_n t + B \sin \omega_n t \quad (5)$$

- The expression for velocity, \dot{u} , is :

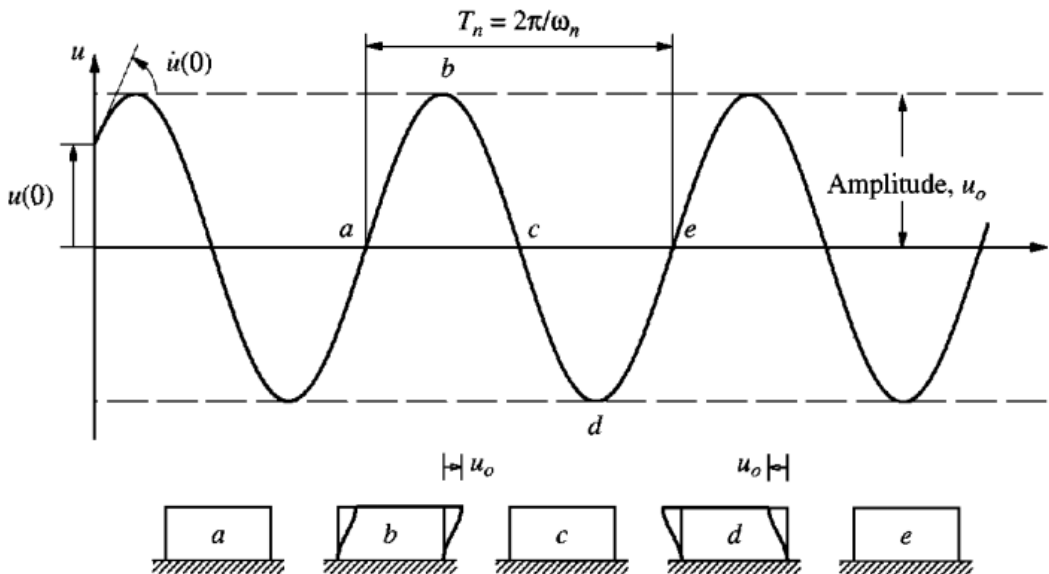
$$\dot{u} = -A \omega_n \sin \omega_n t + B \omega_n \cos \omega_n t \quad (6)$$

- Constant A and B, will be determined based on initial condition (at $t = 0$) :

$$u = u(0) \quad \dot{u} = \dot{u}(0)$$

- Subject to these initial conditions, the solution to the eq. (5) will be :

$$u(t) = u(0)\cos \omega_n t + \frac{\dot{u}(0)}{\omega_n} \sin \omega_n t \quad (7)$$



$$u_0 = \sqrt{[u(0)]^2 + \left[\frac{\dot{u}(0)}{\omega_n}\right]^2}$$

Frequency and Period

- The time required for the undamped system to complete one cycle of free vibration is the natural period of vibration, T_n (in second)

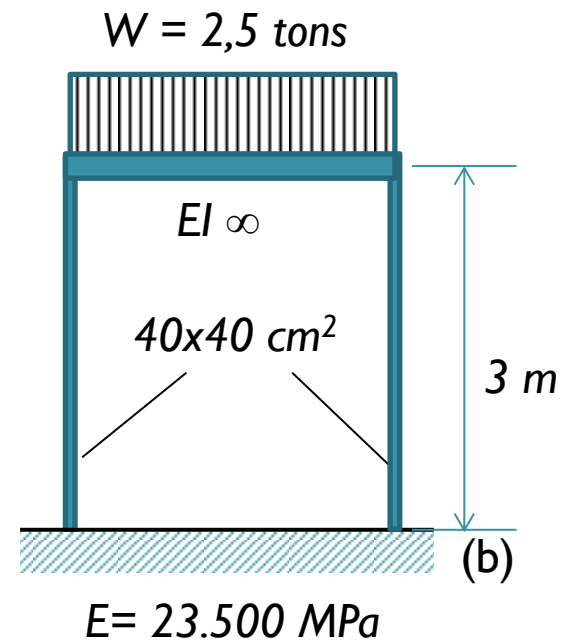
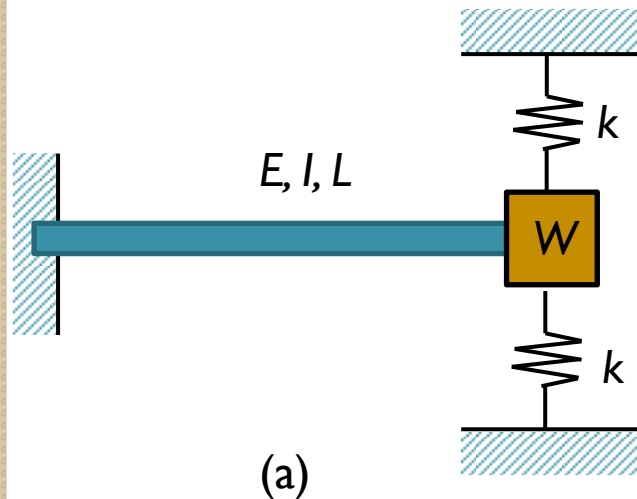
$$T_n = \frac{2\pi}{\omega_n} \quad (8)$$

- A system executes $1/T_n$ cycles in 1 sec. This natural cyclic frequency of vibration (in Hz) is denoted by :

$$f_n = \frac{\omega_n}{2\pi} \quad (9)$$

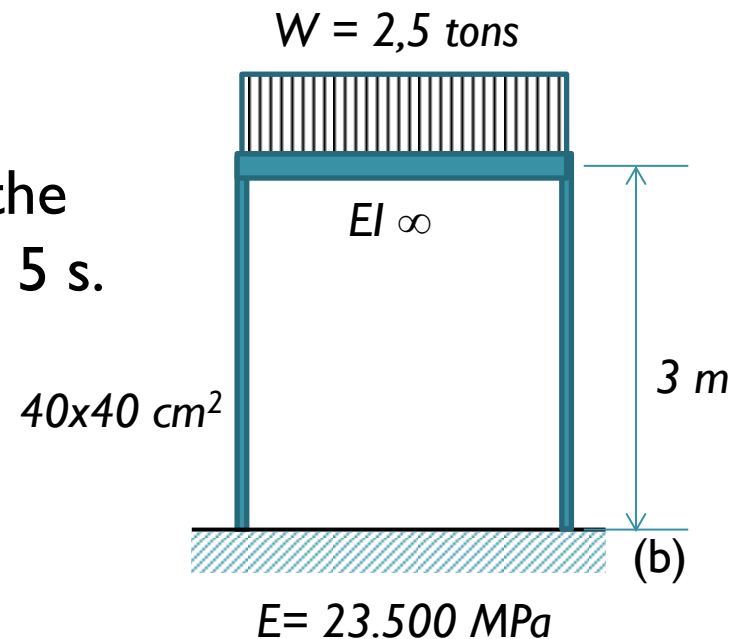
Exercise

- Determine natural frequency and natural period from each system below



Assignment I

- If system (b) have initial condition $u(0) = 3 \text{ cm}$ & $\dot{u}(0) = 20 \text{ cm/s}$
 - a) Determine the displacement at $t = 2 \text{ sec}$
 - b) Plot a time history of displacement response of the system, for $t = 0 \text{ s}$ until $t = 5 \text{ s}$.



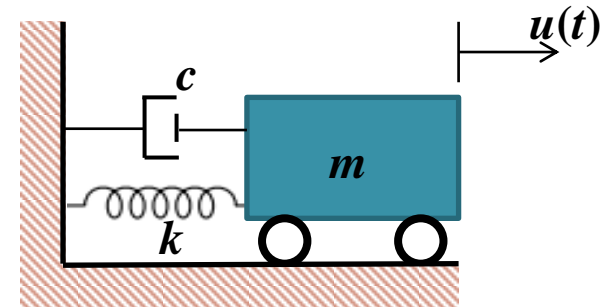
Damped SDoF Free-Vibration

- If damping is present in the system, the differential equation governing free vibration of the systems is :

$$m\ddot{u}(t) + c\dot{u}(t) + ku(t) = 0 \quad (10)$$

- The trial solution to satisfy Eq. (10) is $u = Ce^{pt}$ (11)
- Substitute Eq. (11) to Eq. (10) yield the characteristic eq. :

$$mp^2 + cp + k = 0 \quad (12)$$



- The roots of the quadratic Eq. (12) are :

$$p_{1,2} = -\frac{c}{2m} \pm \sqrt{\left(\frac{c}{2m}\right)^2 - \frac{k}{m}} \quad (13)$$

- Thus the general solution of Eq. (10) is :

$$u(t) = C_1 e^{p_1 t} + C_2 e^{p_2 t} \quad (14)$$

- Three distinct cases may occur depends on the sign under the radical in Eq. (13)

1. Critically Damped System

- For critical damping :

$$c = c_{cr} = 2m\omega_n = 2\sqrt{km} \quad (15)$$

- The expression under the radical in Eq. (13) is equal to zero, and the roots are equal, they are :

$$p_1 = p_2 = -\frac{c_{cr}}{2m} = -\omega_n \quad (16)$$

- The general solution of Eq. (14) is :

$$u(t) = (A + Bt)e^{-\omega_n t} \quad (17)$$

1. Critically Damped System

- Constant A and B, will be determined based on initial condition (at $t = 0$) :

$$u = u(0) \qquad \dot{u} = \dot{u}(0)$$

- Substitute the initial condition, yield :

$$u(t) = e^{-\omega_n t} [u(0) + (\dot{u}(0) + \omega_n u(0))t] \qquad (18)$$

2. Overdamped System

- In an overdamped system, the damping coefficient is greater than the value for critical damping ($c > c_{cr}$)
- EoM of an overdamped system due to initial condition is :

$$u(t) = e^{-\xi\omega_n t} \left[A e^{-\omega'_D t} + B e^{\omega'_D t} \right] \quad (19)$$

- Where :
$$A = \frac{-\dot{u}(0) + (-\xi + \sqrt{\xi^2 - 1})\omega_n u(0)}{2\omega'_D}$$

$$B = \frac{\dot{u}(0) + (-\xi + \sqrt{\xi^2 - 1})\omega_n u(0)}{2\omega'_D}$$

$$\omega'_D = \omega_n \sqrt{\xi^2 - 1}$$

$$\xi = \frac{c}{c_r}$$

- ξ is the **damping ratio** (%)

3. Underdamped System

- When the value of the damping coefficient is less than the critical value ($c < c_{cr}$), the roots of the characteristic eq. (12) are complex conjugates, so that :

$$p_{1,2} = -\frac{c}{2m} \pm i \sqrt{\frac{k}{m} - \left(\frac{c}{2m}\right)^2} \quad i = \sqrt{-1}$$

- Regarding the Euler's equations which relate exponential and trigonometric functions :

$$e^{ix} = \cos x + i \sin x$$

$$e^{-ix} = \cos x - i \sin x$$

3. Underdamped System

- The substitution of the roots p_1 and p_2 into Eq. (14) together with the use of Euler equation, gives :

$$u(t) = e^{-\xi\omega_n t} (A \cos \omega_D t + B \sin \omega_D t) \quad (20)$$

- Where $\omega_D = \omega \sqrt{1 - \xi^2}$
- Using initial condition, $u = u(0) \quad \dot{u} = \dot{u}(0)$

$$u(t) = e^{-\xi\omega_n t} \left(u(0) \cos \omega_D t + \frac{\dot{u}(0) + \xi\omega_n u(0)}{\omega_D} \sin \omega_D t \right) \quad (21)$$

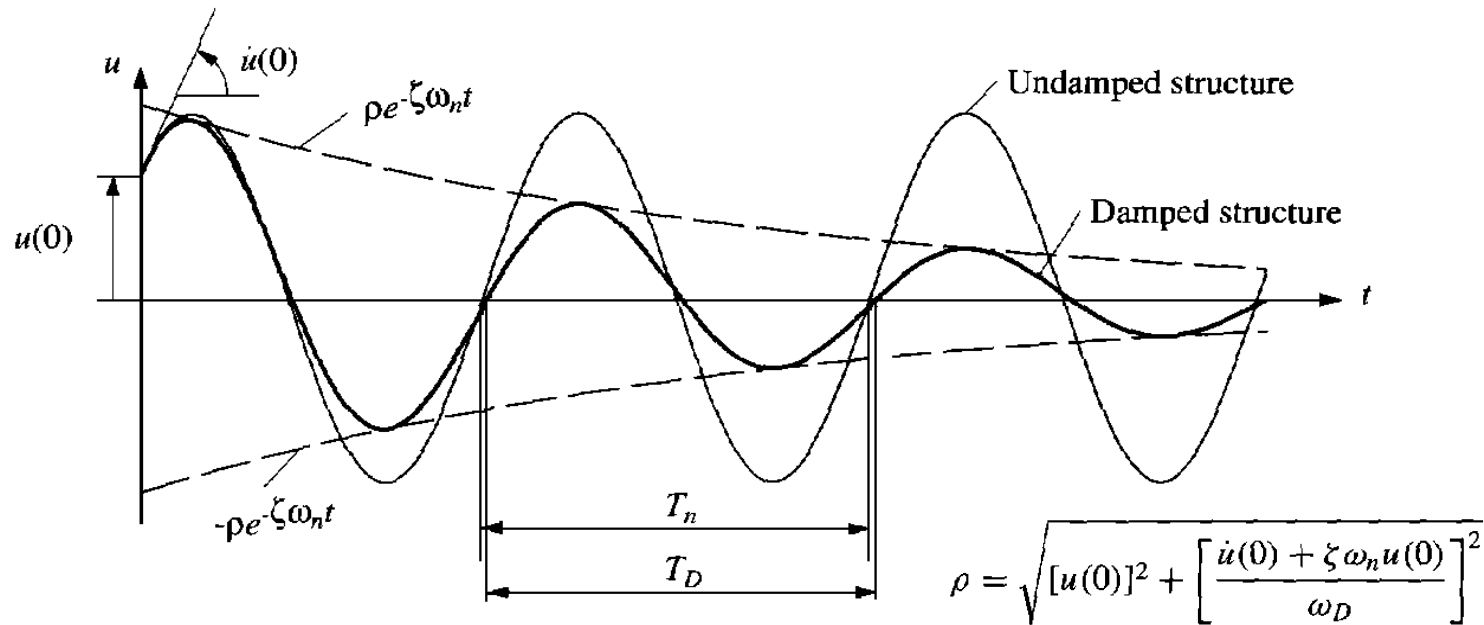


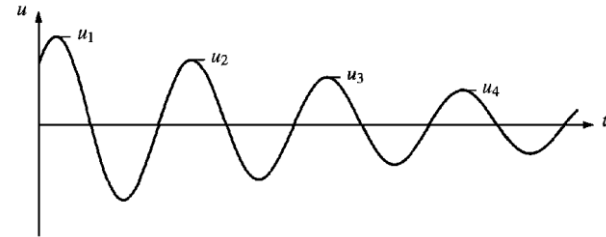
Figure 2.2.2 Effects of damping on free vibration.

Many types of structures, such as buildings, bridges, dams, nuclear power plants, offshore structures, etc., all fall into underdamped system ($c < c_{cr}$), because typically their damping ratio is less than 0,10.

Logarithmic Decrement

- The ratio u_i/u_{i+1} of successive peaks (maxima) is :

$$\frac{u_i}{u_{i+1}} = \exp\left(\frac{2\pi\zeta}{\sqrt{1-\zeta^2}}\right) \quad (22)$$



- The natural logarithm of this ratio, called the logarithmic decrement,

$$\delta = \ln \frac{u_i}{u_{i+1}} = \frac{2\pi\zeta}{\sqrt{1-\zeta^2}} \quad (23)$$

- If ζ is small, this gives an approximate equation : $\delta \approx 2\pi\zeta$

- Over j cycles the motion decreases from u_1 to u_{j+1} . This ratio is given by :

$$\frac{u_1}{u_{j+1}} = \frac{u_1}{u_2} \cdot \frac{u_2}{u_3} \cdot \frac{u_3}{u_4} \dots \frac{u_j}{u_{j+1}} = e^{j\delta} \quad (24)$$

- Therefore,

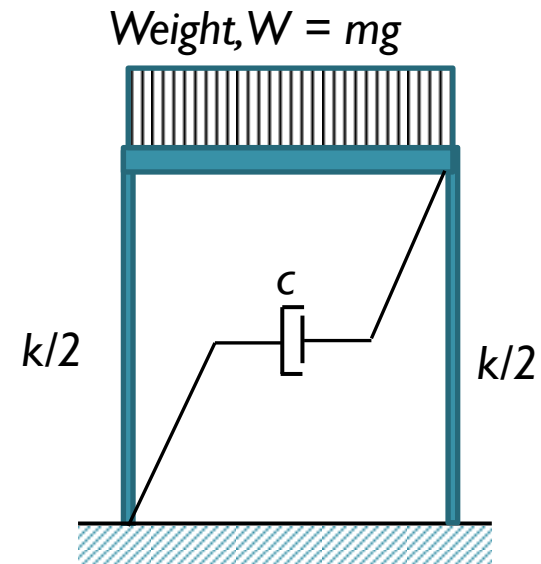
$$\delta = \frac{1}{j} \ln \frac{u_1}{u_{j+1}} \cong 2\pi\xi \quad (25)$$

Exercise

A one-story building is idealized as a rigid girder supported by weightless columns. In order to evaluate the dynamic properties of this structure, a free-vibration test is made, in which the roof system (rigid girder) is displaced laterally by a hydraulic jack and then suddenly released.

During the jacking operation, it is observed that a force of **10 kg** is required to displace the girder **0.508 cm**.

After the instantaneous release of this initial displacement, the maximum displacement on the first return swing is only **0.406 cm** and the period of this displacement cycle is **$T = 1.40$ sec**.



Find :

- W
- f and ω
- δ , ξ , c , ω_D
- u_6

Assignment 2

- If system in the figure have initial condition $u(0) = 3 \text{ cm}$ & $\dot{u}(0) = 20 \text{ cm/s}$
- Plot a time history of displacement response of the system, for $t = 0 \text{ s}$ until $t = 5 \text{ s}$, if :
 - a. $\xi = 5\%$
 - b. $\xi = 100\%$
 - c. $\xi = 125\%$

